MODELLING OF SEABED SCOUR IN SUPPORT OF A MARINE PIPELINE INSTALLATION

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Marine Pipeline Installation



- Marine pipelines can installed using a towed pipeline plough that simultaneously cuts a trench and lowers the pipeline
- Backfill of the trench, if required, can be performed with an additional pass along the pipeline with a backfill plough







Plough is lowered over pipeline







Mechanical arms at front and back of plough lift pipeline off seabed







- Plough blades are lowered into place
- Pipeline is lowered into cradle







Plough is pulled by support vessel, cutting trench and side-casting spoil







- Pipeline slides through cradle as plough is pulled forward
- Plough rides on skids at front, blade at rear







- Pipeline falls into cut trench behind plough
- Trench may be backfilled by a second pass using a backfill plough







- Ploughs tend to be very large
- □ AMP500 is approximately 10 m × 12 m × 22 m and 200 t





Support Vessel Maersk Advancer



Consequently, support vessels are also very large





Problems

- Under certain conditions, the forces required to tow the plough can be very high
- Flow from the vessel props can scour the seabed, leaving the newly-laid pipeline exposed or worse still, spanning an scour hole
 - Softer sediments
 - Shallow water





Propwash-Induced Scour



 Propwash-induced scour holes undermining pipeline are evident in this colourized sonar image





Project Aims

- Develop predictive relationships for near-bed velocities and scour growth as a function of:
 - Vessel characteristics (e.g., speed, thrust, trim, ...)
 - Local conditions (e.g., depth, soil type, ...)
- These relationships would be used to:
 - Plan the operations so as to minimize the issue
 - Enable the personnel on board the tow vessel to modify the operation as required, based on the actual performance observed





Computer Modelling

- Flows
 - 3D Propwash model
- Morphology
 - PTM model
- Dynamically-coupled the models







Flow Model

Propwash model

- 3D velocity fields based on jet flow assumption
- flow spreads and decays with distance from the vessel due to turbulent diffusion
- function of water depth, propeller diameter, spacing, trim and thrust
- dynamically-coupled to PTM







Sediment Model

- Particle Transport Model (PTM)
- Developed by Coldwater
- Funded by two US Army Corps of Engineers Engineering
 Research and Development Center (ERDC) research programs:
 - Coastal Inlets Research Program (CIRP)
 - Dredging Operations and Environmental Research (DOER) Program
- Commercially-available as part of the Surface-water Modeling System (SMS) from Aquaveo





PTM

- PTM uses hydrodynamics from other models
- Multiple input
 - **FE, FD, quadtree, ...**
 - Waves, flows, ...
- PTM was developed for application to dredging and coastal projects
 - dredged material dispersion and fate, sediment pathway and fate, and constituent transport





PTM

- Lagrangian (particle-based) scheme to compute the pathways and fate of sediments
- Typical application method for PTM







PTM

- Eulerian (mesh-based) calculations of bed conditions (e.g. shears, sediment transport, morphology, etc.) are always performed in the background of each run
- Present work illustrates use of these other capabilities
 - Non-cohesive and cohesive sediment transport and morphological evolution







Test Conditions for Present Case

Bed materials

- fine, medium and coarse sand
- soft, medium and hard clay
- Water depth
 - 12 → 24 m
- Vessel speed
 - □ 0 → 400 m/hr

Bollard pull

■ 100 → 200 Te

| Bed | Case | Rate (m/s) | Depth (m) | Bollard Pull (Te) | |
|--------------|------|---------------|--------------|-------------------------|--|
| Fine Sand | A1 | 0 | 12 | 100 | |
| | A3 | 0 | 12 | 200 | |
| | A5 | 0 | 14 | 100 | |
| | A7 | 0 | 14 | 200 | |
| | A9 | 0 | 24 | 200 | |
| | B1 | 200 | 12 | 100 | |
| | B3 | 200 | 12 | 200 | |
| | B4 | 200 | 24 | 200 | |
| Med. Sand | A2 | 0 | 12 | 100 | |
| | A4 | 0 | 12 | 200 | |
| | A6 | 0 | 14 | 100 | |
| | A7 | 0 | 14 | 200 | |
| | A10 | 0 | 24 | 200 | |
| | B2 | 200 | 12 | 100 | |
| Soft Clay | C3 | 0 | 12 | 100 | |
| | C6 | 0 | 24 | 100 | |
| | D3 | 400 | 12 | 100 | |
| Med. Clay | C1 | 0 | 12 | 100 | |
| | C4 | 0 | 24 | 100 | |
| | D1 | 400 | 12 | 100 | |
| Hard Clay | C2 | 0 | 12 | 100 | |
| | C5 | 0 | 24 | 100 | |
| | D2 | 400 | 12 | 100 | |





Maximum Flow Velocities



- Typical flow pattern at the bed
- Note that the vessel has two props





Maximum Flow Velocities



- Peak velocities from all the simulations were used to generate plots of maximum flow velocity at the bed as a function of bollard pull and water depth
- Results are unique to this case as they take into account specific characteristics of the setup
 - Prop size, blade pitch, ...
 - Vessel trim under load
 - ...
 - Chart designed for use by onboard engineers

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Critical Conditions



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Stationary Vessel











































Observations

- As the vessel proceeds along a plough line, a scour hole can develop roughly 50 to 200 m behind the vessel
- Generally, the scouring action of the propwash is backfilled through deposition processes as the ship makes headway
 - Depth of sediment disturbance is significantly greater than the final scour trench depth
- Due to sediment spreading (diffusion), this infilling does not fully equal the scour hole excavation resulting in a net trenching that is shallower than the initial scour hole excavation





Maximum Scour Rate



- Scour rate chart prepared for engineers on board
- To be used in conjunction
 with maximum bed velocity
 chart presented earlier



Maximum Scour Rate and Shear







Summary of Scour Rates

| Bed | Case | Rate (m/s) | Depth (m) | Bollard | 10 min. | 20 min. | 30 min. | Avg. | Max. | Max. |
|-----------------------|------|---------------|--------------|---------|---------|---------|---------|----------|----------|-------|
| | | | | Pull | Scour | Scour | Scour | dz/dt | dz/dt | Scour |
| | | | | (Te) | (cm) | (cm) | (cm) | (cm/min) | (cm/min) | (cm) |
| Fine Sand | A1 | 0 | 12 | 100 | 50.6 | 96.8 | 139.3 | 4.6 | 5.3 | |
| | A3 | 0 | 12 | 200 | 201.6 | 323.3 | 452.5 | 15.1 | 23.7 | |
| | A5 | 0 | 14 | 100 | 33.1 | 62.1 | 87.6 | 2.9 | 3.5 | |
| | A7 | 0 | 14 | 200 | 125.1 | 203.9 | 260.7 | 8.7 | 16.7 | |
| | A9 | 0 | 24 | 200 | 0.5 | 0.9 | 1.4 | 0.05 | 0.05 | |
| | B1 | 200 | 12 | 100 | | | | | | 46.3 |
| | B3 | 200 | 12 | 200 | | | | | | 159.6 |
| | B4 | 200 | 24 | 200 | | | | | | 0.5 |
| Med. A Sand A E | A2 | 0 | 12 | 100 | 13.6 | 26.8 | 39.7 | 1.3 | 1.4 | |
| | A4 | 0 | 12 | 200 | 48.1 | 93.6 | 136.8 | 4.6 | 5.2 | |
| | A6 | 0 | 14 | 100 | 8.8 | 17.3 | 25.4 | 0.8 | 0.9 | |
| | A7 | 0 | 14 | 200 | 35.5 | 66.9 | 95.0 | 3.2 | 3.8 | |
| | A10 | 0 | 24 | 200 | 0.0 | 0.0 | 0.1 | 0.003 | 0.003 | |
| | B2 | 200 | 12 | 100 | | | | | | 11.5 |
| Soft Clay | C3 | 0 | 12 | 100 | 52.4 | 96.9 | 135.7 | 4.5 | 5.7 | |
| | C6 | 0 | 24 | 100 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| | D3 | 400 | 12 | 100 | | | | | | 76.2 |
| Med. Clay | C1 | 0 | 12 | 100 | 8.6 | 16.9 | 25.0 | 0.8 | 0.9 | |
| | C4 | 0 | 24 | 100 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| | D1 | 400 | 12 | 100 | | | | | | 12.2 |
| Hard Clay | C2 | 0 | 12 | 100 | 2.0 | 3.9 | 5.9 | 0.2 | 0.2 | |
| | C5 | 0 | 24 | 100 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| | D2 | 400 | 12 | 100 | | | | | | 2.7 |





Conclusions

- The scour can be backfilled by deposition as the ship makes headway; however, the depth of sediment disturbance is significantly greater than the final scour trench depth
- Irregular tow operations (e.g., occasional high thrust in shallow water) can create conditions wherein large scour holes are generated
- In situations where scour is predicted to be problematic, two tow vessels, one to each side of the trench could be used with each vessel operating at one-half the bollard pull required by the single vessel

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- The width of the predicted scour holes is generally consistent with observed scour holes reported to Coldwater by DeepOcean
 - Of the order of 20 m x 20 m across and 3 to 5 m deep



For More Information

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